

Master Spec Amended for use with Taracon Precast Honed Finished Insulated Precast Panels  
SECTION 03450 - ARCHITECTURAL INSULATED PRECAST CONCRETE WALL PANELS

PART 1 - GENERAL

1.1 RELATED DOCUMENTS

- A. Drawings and general provisions of the Contract, including General and Supplementary Conditions and Division 1 Specification Sections, apply to this Section.

1.2 SUMMARY

- A. This Section includes the following:

1. Architectural precast concrete cladding load bearing units.
2. Insulated, architectural precast concrete units.
3. Panel Finishes
  - a. Honed / Polished Finish
  - b. Light Brush, Brushed, and Hammer Brush, Smooth
  - c. Acid Etch, Sandblast, Exposed Aggregate finished applied after concrete polishing is complete
4. Honed/Polished Finished, architectural precast concrete units.
5. Other Concrete Finishes with the Polisher include light brush, brushed, and hammer brush Other finished that can be incorporated with the Polisher are acid etch, Sandblast, exposed aggregate and smooth

- B. Related Sections include the following:

1. Division 03 Section "Cast-In-Place Concrete" for installing connection anchors in concrete.
2. Division 05 Section "Structural Steel Framing" for furnishing and installing connections attached to structural-steel framing.
3. Division 05 Section "Metal Fabrications" for kickers and other miscellaneous steel shapes.
4. Division 07 Section "Sealants"
5. Division 08 Section "Aluminum Windows" for windows set into architectural precast concrete units.
6. Division 08 Section "Aluminum Entry Doors" for windows set into architectural precast concrete units.
7. Division 08 Section "Coiling / Sectional Overhead Doors" for windows set into architectural precast concrete units.
8. Division 26 Section "Electrical and Lighting" for electrical systems integrated in precast concrete units.

1.3 DEFINITION

- A. Design Reference Sample: Sample of approved architectural precast concrete color, finish and texture, preapproved by Architect.

## 1.4 PERFORMANCE REQUIREMENTS

- A. Structural Performance: Provide architectural precast concrete units and connections capable of withstanding the following design loads within limits and under conditions indicated:
1. Loads: As indicated.
  2. Dead Loads: <Insert applicable dead loads.>
  3. Live Loads: <Insert applicable live loads.>
  4. Wind Loads: <Insert applicable wind loads **or wind-load criteria, positive** and negative for various parts of **building as required** by applicable building code or ASCE 7, **including** basic wind speed, importance factor, exposure category, and pressure coefficient.>
  5. Seismic Loads: <Insert applicable seismic design data **including seismic** performance category, importance factor, use group, seismic design category, seismic zone, site classification, site coefficient, and drift criteria.>
  6. Project Specific Loads: <Insert applicable loads.>
  7. Design framing system and connections to maintain clearances at openings, to allow for fabrication and construction tolerances, to accommodate live-load deflection, shrinkage and creep of primary building structure, and other building movements as follows:
    - a. Upward and downward movement of [1/2 **inch (13 mm)] [3/4 inch (19 mm)] [1 **inch (25 mm)]**].**
  8. Thermal Movements: Provide for in-plane thermal movements resulting from annual ambient temperature changes of [-20 **deg F (-29 deg C)] [120 deg F (48.88 deg C)] <Insert temperature range>**.
  9. Fire-Resistance Rating: Select material and minimum thicknesses to provide [1] [2] <Insert number>-hour fire rating.
  10. Window Washing System: Design precast units supporting window washing system indicated to resist pull-out and horizontal shear forces transmitted from window washing equipment.
  11. Vehicular Impact Loads: Design spandrel beams acting as a vehicular barrier for passenger cars to resist a single [**6000-lb (26.7-kN)] Insert load> service load and [10,000-lb (44.5-kN)] <Insert load> ultimate load applied horizontally in any direction to the spandrel beam, with anchorages or attachments capable of transferring this load to the primary structural frame and the building foundation.**

Design spandrel beams assuming the load to act at a height of **18 inches (460 mm)** above the floor or ramp surface on an area not to exceed **1 sq. ft. (0.93 sq. m)**.

  12. Form-Liner type and design if applicable

## 1.5 ACTION SUBMITTALS

- A. Product Data: For each type of product indicated.
- B. LEED Submittals:
1. Product Data for Credit MR 4: For products having recycled content, documentation indicating percentages by weight of postconsumer and pre-consumer recycled content. Include statement indicating cost for each product having recycled content.
- C. Design Mixtures: For each precast concrete mixture. Include compressive strength and water-

absorption tests.

D. Shop Drawings: Detail fabrication and installation of architectural precast concrete units. Indicate locations, plans, elevations, dimensions, shapes, and cross sections of each unit. Indicate joints, reveals, and extent and location of each surface finish. Indicate details at building corners.

1. Indicate separate face and backup mixture locations and thicknesses.
2. Indicate type and thickness of insulation with details of placement.
3. Indicate welded connections by AWS standard symbols. Detail loose and cast-in hardware and connections.
4. Indicate locations, tolerances, and details of anchorage devices to be embedded in or attached to structure or other construction.
5. Indicate locations, extent, and treatment of dry joints if two-stage casting is proposed.
6. Include plans and elevations showing unit location and sequence of erection for special conditions.
7. Indicate location of each architectural precast concrete unit by same identification mark placed on panel.
8. Indicate relationship of architectural precast concrete units to adjacent materials.
9. Design Modifications: If design modifications are proposed to meet performance requirements and field conditions, submit design calculations and Shop Drawings. Do not adversely affect the appearance, durability, or strength of units when modifying details or materials and maintain the general design concept. Any deviations from the construction documents in panel design, finish or detailing shall be highlighted in the Shop Drawing for Architect review.
10. Indicate locations of Electrical systems to be integrated in the precast wall panels
11. Comprehensive engineering analysis [signed and sealed] [certified] by the qualified professional engineer responsible for its preparation. Show governing panel types, connections, and types of reinforcement, including special reinforcement. Indicate location, type, magnitude, and direction of loads imposed on the building structural frame from architectural precast concrete.

E. Samples:

1. **Prebid** — All Bidders shall submit a 12" x 12" x 2" sample for each type of Architectural finish indicated on exposed surfaces of architectural precast concrete units with their bid indicating their ability to furnish the honed/polished finishes of the design reference samples. Samples shall indicate the aggregate coloring, matrix coloring and luster of honed/polished finish to match the design reference samples.
2. Contract Pre-Production material samples — Submit (3) 12" x 12" x 2" samples for each type of architectural finish indicated on the exposed surfaces of architectural precast concrete panels. Approval will be given to purchase concrete materials and color for production.
3. Production Honed/Polished Color and **Finish** Range Samples — Submit a set of 3 samples for each type of honed/polished finish indicated on the exposed surfaces of architectural precast concrete panels. Samples shall be produced on large 8-Head Bridge-automated Polishing Machine production finishing equipment capable of finishing the largest panel required for the Project . Size shall be min of 48" x 48". The 3 samples of each type shall indicate the expected color and honed/polished finish range for the project for approval.
  - a. One of the Approved samples will be sent to the site for reference while the rest will stay at the plant for reference/quality control.
  - b. Damage part of an exposed-face surface for each finish, color, and texture, and demonstrate adequacy of repair techniques proposed for field repair of surface blemishes.
4. Full Size production Panel — One or more full size production panels shall be presented for viewing at the precast plant for approval prior to start of production. Panels shall demonstrate the color, honed/polished finish, reveal patterns, edge finish, back finish,

insulation details and overall quality of workmanship for approval.

- a. The approved panels will remain at the precast plant for reference of finished quality.
- b. The approved panels may be incorporated into the finished work with the Architect's approval.

## 1.6 INFORMATIONAL SUBMITTALS

- A. Qualification Data: For [Installer] [fabricator] [testing agency].
- B. Welding certificates.
- C. Material Certificates: For the following items, signed by manufacturers:
  1. Cementitious materials.
  2. Reinforcing materials and prestressing tendons.
  3. Admixtures.
  4. Bearing pads and other embeds required for welded steel attachments
  5. Structural-steel shapes and hollow structural sections.
  6. Material Test Reports: For aggregates.
- D. Source quality-control test reports.
- E. Field quality-control test [and special inspection] reports.

## 1.7 QUALITY ASSURANCE

- A. Installer Qualifications: A precast concrete erector qualified and designated by PCI's Certificate of Compliance to erect Category [A (Architectural Systems) for non-load] [S2 (Complex **Structural** Systems) for load]-bearing members.
- B. Installer Qualifications: A precast concrete erector who has retained a "PCI-Certified Field Auditor" to conduct a field audit of a project in same category as this Project before erection of precast concrete and who can produce an Erectors' Post-Audit Declaration.
- C. Fabricator Qualifications: A firm that assumes responsibility for engineering architectural precast concrete units to comply with performance requirements. This responsibility includes preparation of Shop Drawings and comprehensive engineering analysis by a qualified professional engineer.
  1. Participates in PCI's plant certification program [at time of **bidding**] and is designated a PCI-certified plant for Category AA Architectural Cladding and Load Bearing Units
  2. Precast Production Plant shall include 8-head main bridge automated and 8 head bridge automated auxiliary honing/polishing machine with water recycle equipment capable of finishing 15' x 60' panels with a honed/polished finish to match Design Reference Sample. Facilities shall include tilt table for panel handling during polishing. *Small one head floor finishing machines or handheld polishers are not acceptable.*
- D. Testing Agency Qualifications: An independent testing agency, [acceptable to authorities **having jurisdiction,**] qualified according to ASTM C 1077 and ASTM E 329 for testing indicated.
- E. Design Standards: Comply with **ACI 318 (ACI 318M)** and design recommendations of PCI MNL 120, "PCI Design Handbook - Precast and Prestressed Concrete," applicable to types of architectural precast concrete units indicated.

- F. Quality-Control Standard: For manufacturing procedures and testing requirements, quality-control recommendations, and dimensional tolerances for types of units required, comply with PCI MNL 117, "Manual for Quality Control for Plants and Production of Architectural Precast Concrete Products."
- G. Welding: Qualify procedures and personnel according to AWS D1.1/D.1.1M, "Structural Welding Code - Steel"; and AWS D1.4, "Structural Welding Code - Reinforcing Steel."
- H. Calculated Fire-Test-Response Characteristics: Where indicated, provide architectural precast concrete units whose fire resistance has been calculated according to [ACI 216.1/TMS 0216.1, "Standard Method for **Determining Fire** Resistance of Concrete and Masonry Construction Assemblies,"] [PCI MNL 124, "Design for Fire Resistance of Precast Prestressed Concrete,"] and is acceptable to authorities having jurisdiction.
- I. Sample Panels: Approved 12" x 12" material samples, along with a range sample and approved full size panels shall remain at the precast plant facility for reference in approving color, finish, and quality of workmanship during production casting.
- J. Mockups: After range sample approval, but before precast production; when required by contract; construct a full—sized mockup as outlined in the contract drawings. Obtain approval of mockup shop drawings by all trades. Install at the precast plant or on site as directed by the architect to verify all selections and compatibility of architectural materials made under various sample submittals including precast, windows and caulking, to demonstrate aesthetic effects and set quality standards for materials and installation execution. Approval of mockups does not constitute approval of deviations from the Contract Documents unless such deviations are specifically approved by the Architect in writing.
- K. Preconstruction Testing Mockup: Provide a full-size mockup of architectural precast concrete indicated on Drawings for preconstruction testing. Refer to Division [011] [088] < Insert **Division number**> Section "<Insert Section title>" for preconstruction testing requirements.
  - 1. Build preconstruction testing mockup as indicated on Drawings including [aluminum framing, glass, sealants,] <Insert construction> and architectural precast concrete complete with anchors, connections, flashings, and joint fillers.
  - 2. Build preconstruction testing mockup at testing agency facility.
- L. Preinstallation Conference: Conduct conference at Project site to comply with requirements in Division 1 Section "Project Management and Coordination."

## 1.8 DELIVERY, STORAGE, AND HANDLING

- A. Deliver architectural precast concrete units in such quantities and at such times to limit unloading units temporarily on the ground.
- B. Support units during shipment on non-staining shock-absorbing material.
- C. Store units with adequate dunnage and bracing and protect units to prevent contact with soil, to prevent staining, and to prevent cracking, distortion, warping or other physical damage.
- D. Place stored units so identification marks are clearly visible, and units can be inspected.
- E. Handle and transport units in a position consistent with their shape and design in order to avoid excessive stresses which would cause cracking or damage.
- F. Lift and support units only at designated points shown on Shop Drawings.

## 1.9 SEQUENCING

- A. Furnish loose connection hardware and anchorage items to be embedded in or attached to other construction without delaying the Work. Provide locations, setting diagrams, templates, instructions, and directions, as required, for installation.

## PART 2 - PRODUCTS

### 2.1 MANUFACTURERS

- A. Available Fabricators: Subject to compliance with requirements, fabricators offering products that may be incorporated into the Work include, but are not limited to, the following:
- B. Fabricators: Subject to compliance with requirements, provide honed/polished products by one of the following:
  - 1. Taracon Precast; 6189 170<sup>th</sup>  
St. N Hawley, MN 56549

2. Approved equal (approved equal to have 8-head Bridge-automated main and 8-head Bridge-automated auxiliary finishing machine capable of finishing a 15' x 60' panel and approved sample matching the design reference sample).

## 2.2 MOLD MATERIALS

- A. Molds: Rigid, dimensionally stable, non-absorptive material, warp and buckle free, that will provide continuous and true precast concrete surfaces within fabrication tolerances indicated; nonreactive with concrete and suitable for producing required finishes.
  1. Mold-Release Agent: Commercially produced liquid-release agent that will not bond with, stain or adversely affect precast concrete surfaces and will not impair subsequent surface or joint treatments of precast concrete.
- B. Form Liners: Units of face design, texture, arrangement, and configuration [indicated] [to match those used for precast concrete design reference sample]. Furnish with manufacturer's recommended liquid-release agent that will not bond with, stain, or adversely affect precast concrete surfaces and will not impair subsequent surface or joint treatments of precast concrete.

## 2.3 REINFORCING MATERIALS

- A. Recycled Content of Steel Products: Postconsumer recycled content plus one-half of pre-consumer recycled content not less than **[25] [60] <Insert number>** percent.
- B. Reinforcing Bars: ASTM A 615/A 615M, **Grade 60 (Grade 420)**, deformed.
- C. Low-Alloy-Steel Reinforcing Bars: ASTM A 706/A 706M, deformed.
- D. Galvanized Reinforcing Bars: [ASTM A 615/A **615M, Grade 60 (Grade 420)**] [ASTM A 706/A 706M], deformed bars, ASTM A 767/A 767M, Class II zinc coated, hot-dip galvanized[, and chromate wash treated after **fabrication** and bending].
- E. Epoxy-Coated Reinforcing Bars: [ASTM A **615/A 615M, Grade 60 (Grade 420)**] [ASTM A 706/A 706M], deformed bars, [ASTM A 775/A 775M] [or] [ASTM A **934/A 934M**] epoxy coated.
- F. Steel Bar Mats: [ASTM A 184/A 184M, ASTM A **615/A 615M, Grade 60, ASTM A 707/A 706M**] deformed bars to be assembled with clips or ties.
- G. Plain-Steel Welded Wire Reinforcement: ASTM A 185, fabricated from [as-drawn] [galvanized] steel wire into flat sheets.
- H. Deformed-Steel Welded Wire Reinforcement: ASTM A 497/A 497M, flat sheet.
- I. Epoxy-Coated-Steel Wire: ASTM A 884/A 884M, Class A coated, [plain] [deformed], flat sheet, Type [1 bendable] [2 non-bendable] coating.
- J. Supports: Suspend reinforcement from back of mold or use bolsters, chairs, spacers, and other devices for spacing, supporting, and fastening reinforcing bars and welded wire reinforcement in place according to PCI MNL 117.

## 2.4 PRESTRESSING TENDONS

- A. Prestressing Strand: ASTM A 416/A 416M, Grade 270 (Grade 1860), uncoated, 7-wire, low-relaxation strand.
  - 1. Coat unbonded post-tensioning strand with corrosion inhibitor passing ASTM D 1743 and sheath with polypropylene tendon sheathing. Include anchorage devices and coupler assemblies.

## 2.5 CONCRETE MATERIALS

- A. Portland Cement: ASTM C 150, Type I or Type III, gray or Type I White as required to match the design reference sample.
- B. Supplementary Cementitious Materials for backup structural mix:
  - 1. Fly Ash: ASTM C 618, Class C or F, with maximum loss on ignition of 3 percent.
  - 2. Metakaolin Admixture: ASTM C 618, Class N.
  - 3. Silica Fume Admixture: ASTM C 1240, with optional chemical and physical requirement.
  - 4. Ground Granulated Blast-Furnace Slag: ASTM C 989, Grade 100 or 120.
- C. Normal-Weight Aggregates: Except as modified by PCI MNL 117, ASTM C 33, with coarse aggregates complying with Class 5S. Stockpile fine and coarse aggregates for each type of exposed finish from a single source (pit or quarry) for Project.
  - 1. Face-Mixture-Coarse Aggregates: Selected, hard, and durable; free of material that reacts with cement or causes staining; to match selected design reference finish sample and suitable for a honed/polished finish.
    - a. Aggregate Type, Size and Color: [Insert aggregate type here]
      - Fine Aggregates: Selected, natural or manufactured sand of same material

- D. Coloring Admixture: ASTM C 979, synthetic or natural mineral-oxide pigments or colored water-reducing admixtures, temperature stable, and nonfading.
  - 1. Pigment Admixture: [Insert Pigment/color and percentage here]
- E. Water: Potable; free from deleterious material that may affect color stability, setting, or strength of concrete and complying with chemical limits of PCI MNL 117.
- F. Air-Entraining Admixture: ASTM C 260, certified by manufacturer to be compatible with other required admixtures.
- G. Chemical Admixtures: Certified by manufacturer to be compatible with other admixtures and to not contain calcium chloride, or more than 0.15 percent chloride ions or other salts by weight of admixture.
  - 1. Water-Reducing Admixtures: ASTM C 494/C 494M, Type A.
  - 2. Retarding Admixture: ASTM C 494/C 494M, Type B.
  - 3. Water-Reducing and Retarding Admixture: ASTM C 494/C 494M, Type D.
  - 4. Water-Reducing and Accelerating Admixture: ASTM C 494/C 494M, Type E.
  - 5. High-Range, Water-Reducing Admixture: ASTM C 494/C 494M, Type F.
  - 6. High-Range, Water-Reducing and Retarding Admixture: ASTM C 494/C 494M, Type G.
  - 7. Plasticizing and Retarding Admixture: ASTM C 1017/C 1017 M.

## 2.6 STEEL CONNECTION MATERIALS

- A. Carbon-Steel Shapes and Plates: ASTM A 36/A 36M.
- B. Carbon-Steel-Headed Studs: ASTM A 108, AISI 1018 through AISI 1020, cold finished, AWS D1.1/D1.1M, Type A or B, with arc shields and with minimum mechanical properties of PCI MNL 117, Table 3.2.3.
- C. Carbon-Steel Plate: ASTM A 283/A 283M.
- D. Malleable Iron Castings: ASTM A 47/A 47M.
- E. Carbon-Steel Castings: ASTM A 27/A 27M, Grade 60-30 (Grade 415-205).
- F. High-Strength, Low-Alloy Structural Steel: ASTM A 572/A 572M.
- G. Carbon-Steel Structural Tubing: ASTM A 500, Grade B.
- H. Wrought Carbon-Steel Bars: ASTM A 675/A 675M, Grade 65 (Grade 450).
- I. Deformed-Steel Wire or Bar Anchors: ASTM A 496 or ASTM A 706/A 706M.  
Grade A (ASTM F 568 M, Property Class 4.6);
- J. Carbon-steel hex head bolts, studs and nuts, ASTM A 563 (ASTM A 563M); and flat, unhardened steel washers, ASTM F 844.
- K. High-Strength Bolts and Nuts: ASTM A 325 (ASTM A 325M), Type 1, heavy hex steel structural

bolts; heavy hex carbon-steel nuts, **ASTM A 563** (**ASTM A 563M**); and hardened carbon-steel washers, **ASTM F 436** (**ASTM F 436M**).

- L. Zinc-Coated Finish: For exterior steel items [, steel in exterior walls,] and items indicated for galvanizing, apply zinc coating by [hot-dip process according to ASTM A 123/A 123M or ASTM A **153/A 153M**] [electrodeposition according to ASTM B **633**, SC 3, Types 1 and 2].
1. For steel shapes, plates, and tubing to be galvanized, limit silicon content of steel to less than 0.03 percent or to between 0.15 and 0.25 percent or limit sum of silicon and 2.5 times phosphorous content to 0.09 percent.
  2. Galvanizing Repair Paint: High-zinc-dust-content paint with dry film containing not less than 94 percent zinc dust by weight, and complying with DOD-P-21035A or SSPC- Paint 20.
- M. Shop-Primed Finish: Prepare surfaces of nongalvanized steel items, except those surfaces to be embedded in concrete, according to requirements in SSPC-SP 3 and shop-apply [lead- and chromate-free, **rust-inhibitive primer, complying with performance requirements in MPI 79**] [SSPC-Paint 25] according to SSPC-PA 1.
- N. Welding Electrodes: Comply with AWS standards.

## 2.7 STAINLESS-STEEL CONNECTION MATERIALS

- A. Stainless-Steel Plate: ASTM A 666, Type 304, of grade suitable for application.
- B. Stainless—Steel Bolts and Studs: ASTM F 593, Alloy 304 or 316, hex—head bolts and studs; stainless—steel nuts; and flat, stainless—steel washers.
1. Lubricate threaded parts of stainless-steel bolts with an anti-seize thread lubricant during assembly.
- C. Stainless—Steel—Headed Studs: ASTM A 276, with minimum mechanical properties of PC1 MNL 117, Table 3.2.3.

## 2.8 BEARING PADS

- A. Provide one of the following bearing pads for architectural precast concrete units [as recommended by precast fabricator for application]:
1. Elastomeric Pads: AASHTO M 251, plain, vulcanized, 100 percent polychloroprene (neoprene) elastomer, molded to size or cut from a molded sheet, Type A durometer hardness of 50 to 70, ASTM D 2240, minimum tensile strength **22 psi (1.55 MPa)**, ASTM D412.
  2. Oriented, Fiber-Reinforced Elastomeric Pads: Preformed, randomly oriented synthetic fibers set in elastomer. Type A durometer hardness of 70 to 90, ASTM D 2240; capable of supporting a compressive stress of **3000 psi (20.7 MPa)** with no cracking, splitting, or delaminating in the internal portions of pad. Test one specimen for every 200 pads used in Project.
  3. Cotton-Duck-Fabric-Reinforced Elastomeric Pads: Preformed, horizontally layered

cotton-duck fabric bonded to an elastomer; Type A durometer hardness of 80 to 100 ASTM D 2240; complying with AASHTO's "AASHTO Load and Resistance Factor Design (LRFD) Bridge Design Specifications, Division II, Section 18.10.2, or with MIL-C-882E.

4. Frictionless Pads: Tetrafluoroethylene (Teflon), glass-fiber reinforced, bonded to stainless or mild-steel plate, of type required for in-service stress.
5. High-Density Plastic: Multimonomer, non-leaching, plastic strip.

## 2.9 ACCESSORIES

- A. Reglets: Specified in [Division 07 Section "Sheet Metal Flashing and Trim."](#)
- B. Reglets: [PVC extrusions,] [Stainless steel, Type 302 or 304,] [Copper,] felt or fiber filled, or with face opening of slots covered.
- C. Precast Accessories: Provide clips, hangers, plastic or steel shims, and other accessories required to install architectural precast concrete units.

## 2.10 GROUT MATERIALS

- A. Sand-Cement Grout: Portland cement, ASTM C 150, Type I, and clean, natural sand, ASTM C 144 or ASTM C 404. Mix at ratio of 1 part cement to 2-1/2 parts sand, by volume, with minimum water required for placement and hydration.
- B. Nonmetallic, Non-shrink Grout: Premixed, nonmetallic, noncorrosive, non-staining grout containing selected silica sands, portland cement, shrinkage-compensating agents, plasticizing and water-reducing agents, complying with ASTM C 1107, Grade A for dry pack and Grades B and C for flowable grout and of consistency suitable for application within a 30-minute working time.
- C. Epoxy-Resin Grout: Two-component, mineral-filled epoxy resin; ASTM C 881/C 881M, of type, grade, and class to suit requirements.
- D. Sand-Cement Mortar: Portland cement, ASTM C 150, Type I, and clean, natural sand, ASTM C 144. Mix at ratio of 1 part cement to 4 parts sand, by volume, with minimum water required for placement.
- E. [Polyethylene sheet, ASTM D 4397, 6 to 10 mils (0.15 to 0.25 mm) thick].

## 2.11 INSULATED PANEL ACCESSORIES

- A. Molded-Polystyrene Board Insulation: ASTM C 578, Type [I, 0.90 lb/cu. ft. (15 kg/cu. m)] [VIII, 1.15 lb/cu. ft. (18 kg/cu. m)] [II, 1.35 lb/cu. ft. (22 kg/cu. m)]; [square] [ship-lap] edges; with R-value of <Insert valued and thickness of +Insert dimension>.
- B. Extruded-Polystyrene Board Insulation: ASTM C 578, Type [IV, 1.60 lb/cu. ft. (26 kg/cu. m)] [X, 1.30 lb/cu. ft. (21 kg/cu. m)] [VI, 1.110 lb/cu. ft. (29 kg/cu. m)]; [square] [ship-lap] edges; with R-value of <Insert value> and thickness of <Insert dimension>.
- C. Polyisocyanurate Board Insulation: ASTM C 591, Type [I, 1.8 lb/cu. ft. (29 kg/cu. m)] [IV,

2lb/cu. ft. (32 kg/cu. m)] [II, 2.5 lb/cu. ft. (40 kg/cu. m)] unfaced, with R-value of +Insert value> and thickness of <Insert dimension>.

- D. Wythe Connectors: [Glass-fiber and vinyl-ester polymer connectors] [Polypropylene pin connectors] [Stainless-steel pin connectors] [Bent galvanized reinforcing bars or galvanized welded wire trusses] [Cylindrical metal sleeve anchors] manufactured to connect wythes of precast concrete panels.

## 2.12 CONCRETE MIXTURES

- A. Prepare design mixtures for each type of precast concrete required.
- For backup mix, Limit use of fly ash and silica fume to 20 percent of portland cement by weight; limit
1. tkaolin and silica fume to 10 percent of portland cement by weight.
- B. Design mixtures may be prepared by a qualified independent testing agency or by qualified precast plant personnel at architectural precast concrete fabricator's option.
- C. Limit water-soluble chloride ions to maximum percentage by weight of cement permitted by **ACI 315 (ACI 318M)** or PCI MNL 117 when tested according to ASTM C 1218/C 1218M.
- D. Normal-Weight Concrete Mixtures: Proportion [face mixtures] [face and backup **mixtures**] [full-depth mixture] [face and **backup mixtures or full-depth mixtures**, at fabricator's option] by either laboratory trial batch or field test data methods according to ACI 211.1, with materials to be used on Project, to provide normal-weight concrete with the following properties:
1. Compressive Strength (28 Days): 6,000 psi minimum.
  2. Compressive Strength of 5,000 psi at time of honing/polishing
  3. Maximum Water-Cementitious Materials Ratio: 0.38.
- E. Water Absorption: 6 percent by weight or 14 percent by volume, tested according to PCI MNL 117.
- F. Lightweight Concrete Backup Mixtures: Proportion mixtures by either laboratory trial batch or field test data methods according to ACI 211.2, with materials to be used on Project, to provide lightweight concrete with the following properties:
1. Compressive Strength (28 Days): **5000 psi (34.5 MPa)**.
  2. Unit Weight: Calculated equilibrium unit weight of **115 lb/cu. ft. (1542 kg/cu. in)**, plus or minus **3 lb/cu. ft. (48 kg/cu. in)**, according to ASTM C 567.
- G. Add air-entraining admixture at manufacturer's prescribed rate to result in concrete at point of placement having an air content complying with PCI MNL 117.
- H. When included in design mixtures, add other admixtures to concrete mixtures according to manufacturer's written instructions.

## 2.13 MOLD FABRICATION

- A. Molds: Accurately construct molds, mortar tight, of sufficient strength to withstand pressures due to concrete-placement operations and temperature changes and for prestressing and detensioning operations. Coat contact surfaces of molds with release agent before reinforcement is placed. Avoid contamination of reinforcement and prestressing tendons by release agent.
  - 1. Place form liners accurately to provide finished surface texture indicated. Provide solid backing and supports to maintain stability of liners during concrete placement. Coat form liner with form-release agent.
- B. Maintain Molds to provide completed architectural precast concrete units of shapes, lines, and dimensions indicated, within fabrication tolerances specified.

## 2.14 FABRICATION

- A. Connectors, Inserts, Plates, Angles and other Anchorage Hardware: Fabricate to have sufficient anchorage and embedment to comply with design requirements. Accurately position for attachment of loose hardware, and secure in place during precasting operations. Locate anchorage hardware where it does not affect position of main reinforcement or concrete placement.
  - 1. Weld-headed studs and deformed bar anchors used for anchorage according to AWS D1.1/D1.1M and AWS C5.4, "Recommended Practices for Stud Welding."
- B. Furnish loose hardware items including steel plates, clip angles, seat angles, anchors, dowels, cramps, hangers, and other hardware shapes for securing architectural precast concrete units to supporting and adjacent construction.
- C. Cast-in openings larger than **10 inches (250 mm)** in any dimension. Do not drill or cut openings or prestressing strand without Architect's **and Structural Engineer's** approval.
- D. Reinforcement: Comply with recommendations in PCI MNL 117 for fabricating, placing, and supporting reinforcement.
  - 1. Clean reinforcement of loose rust and mill scale, earth, and other materials that reduce or destroy the bond with concrete. When damage to epoxy-coated reinforcing exceeds limits specified in ASTM A 775/A 775M, repair with patching material compatible with coating material and epoxy coat bar ends after cutting.
  - 2. Accurately position, support, and secure reinforcement against displacement during concrete-placement and consolidation operations. Completely conceal support devices to prevent exposure on finished surfaces.
  - 3. Place reinforcement to maintain at least **3/4-inch (19-mm)** minimum coverage. Arrange, space, and securely tie bars and bar supports to hold reinforcement in position while placing concrete. Direct wire tie ends away from finished, exposed concrete surfaces.
  - 4. Place reinforcing steel and prestressing strand to maintain at least **3/4-inch (19-mm)** minimum concrete cover. Increase cover requirements for reinforcing steel to **1-1/2 inches (38 mm)** when units are exposed to corrosive environment or severe exposure conditions. Arrange, space, and securely tie bars and bar supports to hold reinforcement in position while placing concrete. Direct wire tie ends away from finished, exposed concrete surfaces.
  - 5. Install welded wire fabric in lengths as long as practicable. Lap adjoining pieces at least one full mesh spacing and wire tie laps, where required by design. Offset laps of adjoining widths to prevent continuous laps in either direction.

- E. Reinforce architectural precast concrete units to resist handling, transportation, and erection stresses.
- F. Prestress tendons for architectural precast concrete units by either pretensioning or post-tensioning methods. Comply with PCI MNL 117.
  - 1. De-tensioning or post-tensioning of precast, prestressed architectural concrete units assure concrete has reached its indicated minimum design release compressive strength as established by test cylinders cured under same conditions as concrete.
  - 2. De-tension pretensioned tendons either by gradually releasing tensioning jacks or by heat-cutting tendons, using a sequence and pattern to prevent shock or unbalanced loading.
  - 3. If concrete has been heat cured, de-tension while concrete is still warm and moist to avoid dimensional changes that may cause cracking or undesirable stresses.
  - 4. Protect strand ends and anchorages with bituminous, zinc-rich, or epoxy paint to avoid corrosion and possible rust spots.
- G. Comply with requirements in PCI MNL 117 and requirements in this Section for measuring, mixing, transporting, and placing concrete. After concrete batching, no additional water may be added.
- H. Place face mixture to a minimum thickness after consolidation of the greater of 1-1/4 inch (25 mm) or 3.0 times the maximum aggregate size, but not less than the minimum reinforcing cover specified. (full thickness of face mix for insulated panels is preferred for honing/polishing)
- I. Place concrete in a continuous operation to prevent seams or planes of weakness from forming in precast concrete units.
  - 1. Place backup concrete mixture to ensure bond with face-mixture concrete if or where required.
- J. Thoroughly consolidate placed concrete by internal and external vibration without dislocating or damaging reinforcement and built-in items, and minimize pour lines, honeycombing, or entrapped air on surfaces. Use equipment and procedures complying with PCI MNL 117.
  - 1. Place self-consolidating concrete without vibration according to PCI TR-6, "Interim Guidelines for the Use of Self-Consolidating Concrete in Precast/Prestressed Concrete Institute Member Plants."
- K. Comply with PCI MNL 117 for hot- and cold-weather concrete placement.
- L. Identify pickup points of architectural precast concrete units and orientation in structure with permanent markings, complying with markings indicated on Shop Drawings. Imprint or permanently mark casting date on each architectural precast concrete unit on a surface that will not show in finished structure.
- M. Cure concrete, according to requirements in PCI MNL 117, by moisture retention without heat or by accelerated heat curing using low-pressure live steam or radiant heat and moisture. Cure units until compressive strength is high enough to ensure that stripping does not have an effect on performance or appearance of final product.

- N. Discard and replace architectural precast concrete units that do not comply with requirements, including structural, manufacturing tolerance, and appearance, unless repairs meet requirements in PCI MNL 117 and Architect's approval.

## 2.15 INSULATED PANEL CASTING

- A. Cast and screed supported wythe over mold.
- B. Place Insulation boards abutting edges and ends of adjacent boards. Insert wythe connectors and consolidate concrete around connectors according to connector manufacturer's written instructions.
- C. Cast and screed top wythe to meet required finish.

## 2.16 FABRICATION TOLERANCES

- A. Fabricate architectural precast concrete units straight and true to size and shape with exposed edges and corners precise and true so each finished panel complies with PCI MNL 117 product tolerances as well as position tolerances for cast-in items.
- B. Fabricate architectural precast concrete units straight and true to size and shape with exposed edges and corners precise and true so each finished panel complies with the following product tolerances:

- 1. Overall Height and Width of Units, Measured at the Face Exposed to View: As follows:

- a. 10 feet (3 m) or under, plus or minus 1/8 inch (3 mm).
- b. 10 to 20 feet (3 to 6 m), plus 1/8 inch (3 mm), minus 3/16 inch (5 mm).
- c. 20 to 40 feet (6 to 12 m), plus or minus 1/4 inch (6 mm).
- d. Each additional 10 feet (3 m), plus or minus 1/16 inch (1.5 mm).

- 2. Overall Height and Width of Units, Measured at the Face Not Exposed to View: As follows:

- a. 10 feet (3 m) or under, plus or minus 1/4 inch (6 mm).
- b. 10 to 20 feet (3 to 6 m), plus 1/4 inch (6 mm), minus 3/8 inch (10 mm).
- c. 20 to 40 feet (6 to 12 m), plus or minus 3/8 inch (10 mm).
- d. Each additional 10 feet (3 m), plus or minus 1/8 inch (3 mm).

- 3. Total Thickness or Flange Thickness: Plus 1/4 inch (6 mm), minus 1/5 inch (3 mm).

- 4. Rib Thickness: Plus or minus 1/8 inch (3 mm).

- 5. Rib to Edge of Flange: Plus or minus 1/8 inch (3 mm).

- 6. Distance between Ribs: Plus or minus 1/5 inch (3 mm).

- 7. Variation from Square or Designated Skew (Difference in Length of the Two Diagonal Measurements): Plus or minus inch per 72 inches (3 mm per 1830 mm) or inch (13 mm) total, whichever is greater.

- 8. Length and Width of Block-outs and Openings within One Unit: Plus or minus 1/4 inch (6 mm).

- 9. Location and Dimension of Block-outs Hidden from View and Used for HVAC and Utility Penetrations: Plus or minus 3/4 inch (19 mm).

- 10. Dimensions of Haunches: Plus or minus 1/4 inch (6 mm).

11. Haunch Bearing Surface Deviation from Specified Plane: Plus or minus **1/8 inch (3 mm)**.
12. Difference in Relative Position of Adjacent Haunch Bearing Surfaces from Specified Relative Position: Plus or minus **1/4 inch (6 mm)**.
13. Bowing: Plus or minus L/360, maximum **1 inch (25 mm)**.
14. Local Smoothness: **1/4 inch per 10 feet (6 mm per 3 m)**.
15. Warping: **1/16 inch per 12 inches (1.5 mm per 300 mm)** of distance from nearest adjacent corner.

Flushness of Plates: Plus or minus **1/4 inch (6 mm)**.

16. Dimensions of Architectural Features and Rustications: Plus or minus **1/8 inch (3 mm)**.

C. Position Tolerances: Cast in items shall be square and parallel to panel edges unless specifically indicated on architectural drawings.

1. Weld Plates: Plus or minus **1 inch (25 mm)**.
2. Inserts: Plus or minus **1/2 inch (13 mm)**
3. Handling Devices: Plus or minus **3 inches (75 mm)**.
4. Reinforcing Steel and Welded Wire Fabric: Plus or minus **1/4 inch (6 mm)** where position has structural implications or affects concrete cover; otherwise, plus or minus **1/2 inch**
5. Reinforcing Steel Extending out of Member: Plus or minus **1/2 inch (13 mm)** of plan dimensions.
6. Tendons: Plus or minus **1/4 inch (6 mm)**, vertical; plus or minus **1 inch (25 mm)**, horizontal.
7. Location of Rustication Joints: Plus or minus **1/5 inch (3 mm)**.
8. Location of Opening within Panel: Plus or minus **1/4 inch (6 mm)**.
9. Location of Flashing Reglets: Plus or minus **1/4 inch (6 mm)**.
10. Location of Flashing Reglets at Edge of Panel: Plus or minus **1/b inch (3 mm)**.
11. Reglets for Glazing Gaskets: Plus or minus **1/5 inch (3 mm)**.
12. Electrical Outlets, Hose Bibs: Plus or minus **1/2 inch (13 mm)**.
13. Location of Bearing Surface from End of Member: Plus or minus **1/4 inch (6 mm)**.
14. Allowable Rotation of Plate, Channel Inserts, and Electrical Boxes: 2-degree rotation or **1/4 inch (6 mm)** maximum over the full dimension of unit.
15. Position of Sleeve: Plus or minus **1/2 inch (13 mm)**.
16. Location of Window Washer Track or Buttons: Plus or minus **1/8 inch (3 mm)**.

## 2.17 FINISHES

A. Panel faces shall be free of joint marks, grain, and other obvious defects. Corners, including false joints shall be uniform, straight, and sharp. Finish exposed-face surfaces of architectural precast concrete units to match approved [design reference sample] [sample panels] [mockups] and as follows:

1. Design Reference Sample: <Insert description and **identify fabricator** and code number of sample.>
2. Honed Finish: Use 8-Head bridge automated machine on rails to remove up to 1/4" of the surface and hone the finished surface to a min 80 grit honed finish as required to match the design reference sample.
3. Honed Finish: Use continuous mechanical abrasion with fine grit, followed by filling and rubbing procedures to match approved range of samples and full-size mockups.
  - a. 80-120 Grit Matt / Non-Reflective
  - b. 200-400 Grit Satin Finish
  - c. 800-1400 Grit Polished

- d. 1500+ Grit Highly Polished
- 4. Brushed Finish
  - a. Up to 1000 Grit Brush
- 5. Hammer Brushed
  - a. Up to 1000 Grit Brush
- 6. Form Face Finish next to Hone/Polished
  - a. Minimum 5" reveal required between this finish and polished finish
- 7. Acid Etch Finish next to Honed/Polished
  - a. Minimum 5" reveal required between this finish and polished finish
  - b. Light Acid Finish
  - c. Heavy Acid Finish
- 8. Sand Blasted Finish
  - a. Minimum 5" reveal required between this finish and the polished finish
  - b. Light Sand Blast Finish
  - c. Heavy Sand Blast Finish
- 9. Form Liner Finishes
  - a. Minimum 5" reveal required between this finish and the polished finish

B. Finish exposed edge return surfaces of architectural precast concrete units to match face-surface finish as shown on contract drawings.

C. Finish exposed back surfaces of architectural precast concrete units by smooth, steel-trowel finish.

D. Finish unexposed surfaces of architectural precast concrete units by float finish.

## 2.18 SOURCE QUALITY CONTROL

- A. Quality-Control Testing: Test and inspect precast concrete according to PCI MNL 117 requirements. If using self-consolidating concrete, also test and inspect according to PCI TR-6, "Interim Guidelines for the Use of Self-Consolidating Concrete in Precast/Prestressed Concrete Institute Member Plants."
- B. Owner to employ an independent testing agency to evaluate architectural precast concrete quality-control and testing methods.
  - 1. Allow Owner's testing agency access to material storage areas, concrete production equipment, concrete placement, and curing facilities. Cooperate with Owner's testing agency and provide samples of materials and concrete mixtures as may be requested for additional testing and evaluation.
- C. Strength of precast concrete units will be considered deficient if units fail to comply with **ACI 315 (ACI 318M)** requirements for concrete strength.

- D. Testing: If there is evidence that strength of precast concrete units may be deficient or may not comply with **ACI 318 (ACI 318M)** requirements, precaster will employ an independent testing agency to obtain, prepare, and test cores drilled from hardened concrete to determine compressive strength according to ASTM C 42/C 42M.
1. A minimum of three representative cores will be taken from units of suspect strength, from locations directed by Architect.
  2. Cores will be tested in an air-dry condition.
  3. Strength of concrete for each series of 3 cores will be considered satisfactory if average compressive strength is equal to at least 85 percent of 28-day design compressive strength and no single core is less than 75 percent of 28-day design compressive strength.
  4. Test results will be made in writing on same day that tests are performed, with copies to Architect, Contractor, and precast concrete fabricator. Test reports will include the following:
    - a. Project identification name and number.
    - b. Date when tests were performed.
    - c. Name of precast concrete fabricator.
    - d. Name of concrete testing agency.
    - e. Identification letter, name, and type of precast concrete unit(s) represented by core tests; design compressive strength; type of break; compressive strength at breaks, corrected for length-diameter ratio; and direction of applied load to core in relation to horizontal plane of concrete as placed.
- E. Patching: If core test results are satisfactory and precast concrete units comply with requirements, clean and dampen core holes and solidly fill with precast concrete mixture that has no coarse aggregate, and finish to match adjacent precast concrete surfaces.

## PART 3 - EXECUTION

### 3.1 EXAMINATION

- A. Examine supporting structural frame or foundation and conditions for compliance with requirements for installation tolerances, true and level bearing surfaces, and other conditions affecting performance.
- B. Proceed with installation only after unsatisfactory conditions have been corrected.
- C. Do not install precast concrete units until supporting cast-in-place building structural framing attained minimum allowable design compressive strength or supporting steel or other structure is complete.

### 3.2 INSTALLATION

- A. Install clips, hangers, bearing pads, and other accessories required for connecting architectural precast concrete units to supporting members and backup materials.

- B. Erect architectural precast concrete level, plumb, and square within specified allowable tolerances. Provide temporary supports and bracing as required to maintain position, stability, and alignment as units are being permanently connected.
1. Install temporary steel or plastic spacing shims or bearing pads as precast concrete units are being erected. Tack weld steel shims to each other to prevent shims from separating.
  2. Maintain horizontal and vertical joint alignment and uniform joint width as erection progresses.
  3. Remove projecting lifting devices and grout fill voids within recessed lifting devices flush with surface of adjacent precast surfaces when recess is exposed.
  4. Unless otherwise indicated, maintain uniform joint widths of **3/4 inch (19 mm)**.
- C. Connect architectural precast concrete units in position by bolting, welding, grouting, or as otherwise indicated on Shop Drawings. Remove temporary shims, wedges, and spacers as soon as practical after connecting and grouting are completed.
1. Do not permit connections to disrupt continuity of roof flashing.
- D. Welding: Comply with applicable AWS D1.1/D1.1M and AWS D1.4 for welding, welding electrodes, appearance, quality of welds, and methods used in correcting welding work.
1. Protect architectural precast concrete units and bearing pads from damage by field welding or cutting operations, and provide noncombustible shields as required.
  2. Welds not specified shall be continuous fillet welds, using no less than the minimum fillet as specified by AWS.
  3. Clean weld-affected metal surfaces with chipping hammer followed by brushing and apply a minimum **4.0-mil- (0.1-mm-)** thick coat of galvanized repair paint to galvanized surfaces according to ASTM A 780.
  4. Clean weld-affected metal surfaces with chipping hammer followed by brushing and reprime damaged painted surfaces.
  5. Remove, reweld, or repair incomplete and defective welds.
- E. At bolted connections, use lock washers, tack welding, or other approved means to prevent loosening of nuts after final adjustment.
1. Where slotted connections are used, verify bolt position and tightness. For sliding connections, properly secure bolt but allow bolt to move within connection slot. For friction connections, apply specified bolt torque and check 25 percent of bolts at random by calibrated torque wrench.
- F. Grout connections where required or indicated. Retain grout in place until it can support itself. Pack spaces with stiff grout material, tamping until voids are completely filled. Place grout to finish smooth, level, and plumb with adjacent concrete surfaces. Keep grouted joints damp for not less than 24 hours after initial set. Promptly remove grout material from exposed surfaces before it affects finishes or hardens.

### 3.3 ERECTION TOLERANCES

- A. Erect architectural precast concrete units level, plumb, square, true, and in alignment without exceeding the noncumulative erection tolerances of PCI MNL 117, Appendix I.

- B. Erect architectural precast concrete units level, plumb, square, and true, without exceeding the following noncumulative erection tolerances:
1. Plan Location from Building Grid Datum: Plus or minus **1/2 inch (13 mm)**.
  2. Plan Location from Centerline of Steel: Plus or minus **1/2 inch (13 mm)**.
  3. Top Elevation from Nominal Top Elevation: As follows:
    - a. Exposed Individual Panel: Plus or minus **1/4 inch (6 mm)**.
    - b. Non-Exposed Individual Panel: Plus or minus **1/2 inch (13 mm)**.
    - c. Exposed Panel Relative to Adjacent Panel: **1/4 inch (6 mm)**.
    - d. Non-Exposed Panel Relative to Adjacent Panel: **1/2 inch (13 mm)**.
  4. Support Elevation from Nominal Support Elevation: As follows:
    - a. Maximum Low: **1/2 inch (13 mm)**.
    - b. Maximum High: **1/4 inch (6 mm)**.
  5. Maximum Plumb Variation over the Lesser of Height of Structure or **100 Feet (30 m)**: **1 inch (25 mm)**.
  6. Plumb in Any **10 Feet (3 m)** of Element Height: **1/4 inch (6 mm)**.
  7. Maximum Misalignment of Matching Edges: **1/4 inch (6 mm)**.
  8. Joint Width (Governs over Joint Taper): Plus or minus **1/4 inch (6 mm)**.
  9. Maximum Joint Taper: **3/8 inch (10 mm)**.
  10. Joint Taper in **10 Feet (3 m)**: **1/4 inch (6 mm)**.
  11. Maximum Jog in Alignment of Matching Faces: **1/4 inch (6 mm)**.
  12. Differential Bowing or Camber, as Erected, between Adjacent Members of Same Design: **1/4 inch (6 mm)**.
  13. Opening Height between Spandrels: Plus or minus **1/4 inch (6 mm)**.

### 3.4 FIELD QUALITY CONTROL

- A. Special Inspections: [Owner will engage] [Engage] a qualified special inspector to perform the following special inspections and prepare reports:
1. Erection of precast concrete members.
  2. <Insert special inspections.>
- B. Owner will engage a qualified testing agency to perform tests and prepare test reports.
- C. Field welds will be subject to visual inspections and nondestructive testing according to ASTM E 165 or ASTM E 709. High-strength bolted connections will be subject to inspections.
- D. Testing agency will report test results promptly and in writing to Contractor and Architect.
- E. Repair or remove and replace work where tests and inspections indicate that it does not comply with specified requirements.
- F. Additional testing and inspecting, at Contractor's expense, will be performed to determine compliance of replaced or additional work with specified requirements.

### 3.5 REPAIRS

- A. Repair architectural precast concrete units **for approval by the** Architect. The Architect reserves the right to reject repaired units that do not comply with **contract requirements and approved full size samples and** mockups.
- B. Mix patching materials and repair units so cured patches blend with color, texture, and uniformity of adjacent exposed surfaces and show no apparent line of demarcation between original and repaired work, when viewed in typical daylight illumination from a distance of **20 feet (6 m)**.
- C. Prepare and repair damaged galvanized coatings with galvanizing repair paint according to ASTM A 780.
- D. Wire brush, clean, and paint damaged prime-painted components with same type of shop primer.
- E. Remove and replace damaged architectural precast concrete units when repairs do not comply with requirements.

### 3.6 CLEANING

- A. Clean surfaces of precast concrete units exposed to view.
- B. Clean mortar, plaster, fireproofing, weld slag, and other deleterious material from concrete surfaces and adjacent materials immediately.
- C. Clean exposed surfaces of precast concrete units after erection and completion of joint treatment to remove weld marks, other markings, dirt, and stains.
  - 1. Perform cleaning procedures, if necessary, according to precast concrete fabricator's recommendations. Clean soiled precast concrete surfaces with detergent and water, using stiff fiber brushes and sponges, and rinse with clean water. Protect other work from staining or damage due to cleaning operations.
  - 2. Do not use cleaning materials or processes that could change the appearance of exposed concrete finishes or damage adjacent materials.
  - 3. Use cleaning processes that have been tested for use on honed/polished finishes to maintain the quality of finish noted on the approved samples and mockups.

END OF SECTION 034500